MEASUREMENTS OF HYDRAULIC CONDUCTIVITY USING SLUG TESTS IN COMPARISON TO EMPIRICAL CALCULATIONS FOR TWO STREAMS IN THE PACIFIC NORTHWEST, USA

By

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To the Faculty of Washington State University:

The members of the Committee appointed to examine the thesis of

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Measurements of Hydraulic Conductivity Using Slug Tests in Comparison to Empirical Calculations for Two Streams in the Pacific Northwest, USA

Abstract

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Seasonal water shortages caused by natural climate variations coupled with intensified agricultural production and municipal growth demands can be highly detrimental to aquatic habitats. Two streams, Bertrand and Fishtrap Creeks, located in Washington State, Pacific Northwest, USA, are examples of important anadromous fish habitats impacted by seasonal water shortages. This study was aimed at elucidating streambed hydraulic conductivity (*K*) and the patterns of spatial variation for the aforementioned Bertrand and Fishtrap Creeks. Detailed field investigations were carried out and two approaches were adopted to determine *K* values. Instream slug tests following the Bouwer and Rice method were performed to obtain *K* for two depths at multiple sites in each stream. Additionally, samples were taken at the surface and subsurface of the streambed to estimate *K* using four commonly applied empirical formulas, i.e., the Hazen, Harleman, Krumbien and Monk, and Kozeny-Carmen equations.

Comparison of results from the instream slug tests and empirical calculations showed that Hazen and Harleman equations produced results of *K* similar to those from the slug tests whereas the Krumbien and Monk and the Kozeny-Carmen equation overestimated the *K* values of the streambed material. For the future, we recommend the slug test method if accuracy and reliability of the *K* values are desired as in a detailed study, such as modeling surface- and ground-water

interaction. If *K* is not a dominant factor, then the empirical methods would be a more costeffective approach.

- APPENDICES 38 Appendix A: Calculations of *K* from the instream slug tests, Bertrand Creek and Fishtrap Creek, Washington State, USA.
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1. INTRODUCTION

Seasonal water shortages caused by natural climate variations coupled with intensified agricultural production and municipal growth demands can be highly detrimental to aquatic habitats. As our knowledge and understanding of ecosystem water requirements increases, there is a growing concern over the adequacy of water resources for sustaining stream habitats. This is particularly true for streams in the Pacific Northwest, USA, where flows during dry summer periods are sustained by ground-water base flows that are subject to reduction due to increased ground-water pumping. Recognizing this concern, management options, such as changing from direct surface-water diversions to ground-water withdrawals in order to temporarily mitigate low stream flows and to protect anadromous fish species, have recently been proposed (Washington State University Cooperative Extension, 2005). However, to evaluate whether or not these efforts maintain adequate flow for fish habitat throughout the year, a better understanding of the interaction between ground water and surface water is imperative. Such understanding in turn provides vital information for developing sound strategies for managing available water resources.

Hydrologists and ecologists have increasingly recognized that subsurface properties of natural streambeds have a high degree of spatial variability (Stanford and Ward, 1993; Boulton, 1998; Calver, 2001). Packman showed that the spatial variability of the interaction zone, also referred to as the hyporheic zone, directly affects ground- and surface-water exchange (Packman and Salehin, 2003; Packman et al., 2004; Leek, 2006). Furthermore, studies have shown that streambed characteristics, such as bed material, thickness, topography, and channel curvature, influence the streambed hydraulic properties and thus water movement (Packman et al., 2004). Studies investigating physical streambed properties, including the material composition and

compaction, are essential in determining hydraulic conductivity (*K*) values for predicting groundand surface-water interchange (Packman et al., 2004).

K has traditionally been determined through empirical methods, among which are the commonly used relationships by Hazen (1892), Harleman et al. (1963), Krumbein and Monk (1943), and Kozeny-Carman (1972). These methods are primarily based on grain-size distribution, porosity, or the combination of the two, of geological materials, and were developed for specific applications, such as sand dams (Hazen, 1911) or petroleum fields (Domenico and Schwartz, 1998). Hazen (1892) submitted the original equation using representative grain size to determine *K*. Harleman et al. (1963) adapted Hazen's equation for broader uses to porous media that are generally isotropic by using a smaller empirical coefficient. Another approach was developed by Krumbein and Monk (1943) who derived permeability and thus *K* from grain-size distribution. The Kozeny-Carman equation was a modification of the Kozeny's (1927) original equation by Carman (1937) who modifyed the geometric shape coefficient of the particles (Bear,1972). The Kozeny-Carman equation uses both grain-size distribution and porosity in obtaining *K* (Fetter, 2001).

Slug tests have also been used for determining *K*, in particular for aquifers (Hvorslev, 1951; Freeze and Cherry, 1979). A slug test uses a small observation well inserted into the geological material where the rate at which the water level rises or falls is measured (Bouwer, 1976). Although originally designed for unconfined aquifers, slug tests have been used to determine *K* of streambed materials. Hanrahan et al. (2005) conducted instream slug tests to estimate *K* ranges that characterize the physical habitat and its effect on the life cycle of salmon in Hells Canyon of the Snake River in the state of Idaho, USA. Leek (2006) used slug tests to determine the spatial variation of *K* and thus the heterogeneous hydraulic properties of streambeds and potential egg survival rates in a representative stream of the Pacific Northwest, USA.

Many streams in the Pacific Northwest are experiencing low stream-water levels for salmon spawning during the dry season of June–September due to high water-use demands. Local agricultural practices directly impact ground- and surface-water levels because of irrigation diversions. In areas with irrigated agricultural production, irrigation application rates often govern up- and down-welling of a stream (Packman and Salehin, 2003; Packman et al., 2004). To establish sound water resource management plans, a better understanding of the relationship between local water uses and available water resources is needed. The major goal in local water resource management is to develop practices that maintain adequate water levels in the streams while allowing withdrawals for agricultural production. A cost-effective approach is to develop models of surface- and ground-water interaction. The first step is determining the spatial variation in streambed *K* values. The hypothesis is that empirical equations may be used to adequately predict streambed *K* values compared to the more time-consuming and costly approach of instream measurements, e.g., slug tests.

The main objectives of this study were:

- (1) to measure hydraulic conductivity values and the spatial variability along two Pacific Northwest streams using slug tests,
- (2) to evaluate four empirical equations for estimating hydraulic conductivity, and
- (3) to compare the hydraulic conductivity determined using the four afore mentioned empirical methods to the values determined by the slug tests.

This study, with detailed field investigation and statistical analyses, will provide valuable information about streambed hydraulic conductivity values and the patterns of spatial variation in the two study streams as well as those within the study region. Comparison of results from the instream slug tests and empirical calculations will allow us to identify an approach to determining

K that is both reliable and cost-effective.

2. STUDY AREA AND METHODS

2.1 Study Site

Bertrand and Fishtrap Creeks, located in Northwest Washington State, USA, are two important areas that are in need of a sound management plan to balance conflicting needs for water resources in each watershed. The two creeks are tributaries to the Nooksack River located in Whatcom County, Washington (Figure 1). The Nooksack River is a major river in the region, and discharges into the Pacific Ocean via the Strait of Georgia. The USA-Canada border intersects both watersheds with approximately 54% of the Bertrand Creek watershed and 61% of the Fishtrap Creek watershed located in Canada. The study site consisted of 10-km reaches of the Bertrand and Fishtrap Creeks from the Canadian boarder to the Nooksack River.

The primary uses for these creeks are agricultural irrigation as well as salmon spawning and rearing, although tributary ground water is also used for domestic and municipal supplies. The local climate includes warm, dry summers and mild, rainy winters. The mean annual temperature is 9 °C (48 °F), with the maximum monthly temperature of 22 °C (71 °F) occurring in July and August and minimum monthly temperature -1 °C (31 °F) in January (NOAA NCDC, 2007)(Appendix G).

Figure1. Study site map.

The long-term (1949–2001) mean annual precipitation is 90 cm yr⁻¹ (35 in yr⁻¹), with the majority (82%) of rainfall in fall, winter and spring, and the remaining (18%) in summer (NOAA NCDC, 2007). The regional soils have been developed since the Fraser Glaciation 18,000 years ago, including Lynden sandy loam, Hale silt loam, and Tromp loam (Cox and Kahle, 1999). Both creeks are located on gentle landscapes, with a slope gradient for Bertrand and Fishtrap Creeks being 0.26% and 0.24% respectively, as measured in this study.

The area was originally populated primarily with cedar, hemlock, and douglas fir, but was steadily changed to agricultural use in the last century (Cox and Kahle, 1999). The flat terrain of the region supports dairy and berry farming, making Whatcom County one of the largest producers in the state for raspberries, blueberries, and dairy products (Cox and Kahle, 1999).

For the purpose of this study the stream reaches were divided into cross-sections (referred to as 'sites') of approximately one every mile progressing downstream (Figure 2). Accessibility to the sites was limited and permission from land owners was required. As a result, Bertrand Creek contained seven sampling sites and Fishtrap Creek contained six sampling sites.

2.2 Field Testing

At each site, instream slug tests for in-situ determination of streambed hydraulic conductivity were conducted. Grab samples of streambed material for particle-size analysis and core samples for both particle-size analysis and porosity measurements were collected.

Figure 2. Locations of slug test sites. B and F stand for Bertrand Creek and Fishtrap Creek, respectively.

2.2.1 Slug tests

At each site, two locations roughly 0.5 m apart transverse to the stream flow were identified. At each location, a mini-piezometer was manually driven below the streambed to the depths of 0.5-m and 1.0-m sequentially. At each depth the slug test was replicated three times, producing a total of 12 slug tests at each site.

Construction of piezometers and the instream slug tests were primarily following Leek (2006). For completeness, adapted excerpts from Leek (2006) describing the assembly and field test procedures are included below. The piezometers were constructed of 15-cm long, commercial-grade well-screen (Johnson Screens™) welded at the lower end of the 12-cm drive point and at the upper end to a galvanized steel pipe with an outside diameter of 4 cm. Each piezometer was installed using a steel drive rod to penetrate the streambed. A control manifold was threaded to the top of the piezometer before each slug test and kept above the water surface during the test, allowing pressurization and rapid release of the pressure to facilitate the test. Once the piezometer and manifold were in place, a pressure transducer (Model PS-9805, Instrumentation Northwest, Kirkland, WA, USA) was lowered into the piezometer until it reached the bottom of the screen section. The pressure transducer was then lifted 10 cm off the bottom to avoid potential interference from fine silt (Figure 3).

The probe communicated to a Campbell Scientific CR10X data logger (Campbell Scientific, Logan, UT, USA) located on-shore. The data logging was monitored via a laptop computer. The slug test was conducted by forcing the water level in the piezometer down with air pressure, releasing the pressure rapidly, and recording the rate of head recovery. Data from the slug test perturbation were collected at 0.25-s time steps. All slug tests were conducted during the end of July and the beginning of August 2006.

Fig. 3. Piezometer schematic. Drawdown is measured using a pressure transducer (black dot) connected to a data logger.

2.2.2 Soil sampling

Three grab samples, each about 500 g, were taken from the surface of the streambed at each site. The grab samples were sealed in bags for transport and subsequent laboratory analysis of particle-size distribution was conducted. Core samples, 3 cm long, were taken from the streambed surface to about 45 cm and roughly 0.25 m from the location of the slug test towards the stream bank at each site. The samples used for determining the particle-size distribution as well as the porosity of the subsurface material were those from 15 cm below the streambed surface. These samples were cut into 3-cm segments and those regarded unrepresentative (e.g., containing large voids or pebbles) were discarded. The remaining samples were capped and wrapped with tissue and stored in soil cans for transport back to the laboratory.

2.3 *Slug Test Analysis*

The rate of recovery in conjunction with the geometry of the piezometer was used to solve for *K* following Bouwer and Rice (1976)

$$
K = \frac{r^2 \ln(R_e/R)}{2L_e} \frac{1}{t} \ln\left(\frac{H_o}{H_t}\right)
$$
 (1)

where *K* is in m s⁻¹, *r* is the inside radius of the well casing (m), L_e is the length of the well screen (m), R_e/R is the ratio of the distance away from the well over which the average *K* is being measured to the radius of the screen's outside diameter. H_0 is the drawdown (m) at time $t = 0$ (s), *Ht* is the drawdown at time *t* (s). A regression relationship was determined from the fitted exponential trend line to the raw data of the initial head H_0 over the rising head H_t on a semi-log plot (Appendix A).

2.4 Particle-Size Analysis

A 100-g sample was randomly collected from each grab sample and oven-dried for 24 hrs at 107 °C following standard procedures of the USDA (Klute, 1986). The sample was then cooled in a desiccator and run through a RowtapTM sieve shaker. The sieves in the shakers were arranged in order of largest to smallest following the sieve distribution recommendations of the USDA (Klute, 1986). The sieves were weighed before each run. The soil sample was poured into the set of sieves. The sieve shaker was then run for five minutes. Each sieve was weighed again and the difference between the initial weight and the final weight was calculated to obtain the particle-size distribution.

The 3-cm samples used for estimating the porosity of the subsurface materials were those closest to the 0.5-m depth for comparison of *K* estimations from the empirical Kozeny-Carman equation and slug tests. Dry bulk density was determined by oven-drying the samples for 24 hrs at 107 °C and weighing the dried samples and measuring sample volumes. The porosity was then determined from the dry bulk density as

$$
f = 1 - \frac{\rho_b}{\rho_s} \tag{2}
$$

where *f* is porosity, ρ_b is dry bulk density (g cm⁻³), and ρ_s is the particle density, taken as 2.65 g cm^{-3} (Hillel, 1982).

2.5 Empirical Estimation of K

The Hazen (1911) equation is given by

$$
K = C(d_{10})^2 \tag{3}
$$

where *K* is the hydraulic conductivity in cm s⁻¹, *C* is an empirical coefficient taken as 100 (cm s)⁻¹

for fine to medium sized sand particles, and d_{10} is the effective diameter compared to which 10 percent by weight of the particles are finer (cm).

The Harleman et al. (1963) equation takes the form of

$$
K = k \left(\frac{\rho_w g}{\mu} \right) \tag{4}
$$

$$
k = (6.54 \times 10^{-4}) d_{10}^2 \tag{5}
$$

where k is the permeability (cm²), d_{10} is as previously defined, ρ_w is the density of water (1000 kg (m^{-3}) , *g* is the acceleration due to gravity (9.81 m s⁻²), and μ is the viscosity of water (0.001002 N s m^{-2} at 20 °C)

The Krumbein-Monk (1943) equation is described by

$$
k = 760d^2e^{-1.31\sigma}
$$
 (6)

$$
K = k(1.04 \times 10^3) \tag{7}
$$

where *k* is permeability (darcys), *d* is the geometric mean diameter (mm), and σ is the log standard deviation of the geometric mean diameter.

The Kozeny-Carman (1972) equation is

$$
K = \left(\frac{\rho_w g}{\mu}\right) \frac{n^3}{(1-n)^3} \left(\frac{d_m^2}{180}\right)
$$
 (8)

where d_m is the geometric mean diameter (cm), *n* is the porosity of the sample, and ρ_w , *g* and μ are as previously defined.

For the surface samples only the first three methods were applied to estimate *K* because the

core sampling procedure used for the subsurface was not possible for the surface material and therefore porosity could not be determined. For the subsurface materials all four empirical methods were applied to determine *K* from single samples.

2.6 Statistical Analyses

Analysis of variance (ANOVA) tests were preformed at a significance level α =0.05 using the General Linear Model (GLM) in SAS (SAS Institute Inc., 2005) to compare the means of *K* (1) from the slug tests at 0.5-m and 1.0-m depths within each reach and between the two reaches, (2) from the different empirical formulas for the surface and subsurface samples within each reach, respectively, (3) from the Hazen method between the two reaches, for the surface and subsurface, respectively, and (4) from both the slug test and the empirical methods for the subsurface. Singlereplicate ANOVA tests were performed wherever no replicates were available, including all comparisons made for the means of *K* from the empirical formulas for the subsurface.

3. RESULTS AND DISCUSSION

3.1 Slug Test

The slug tests yielded satisfactory results for both the 0.5-m and 1.0-m depths for five of the seven sites in Bertrand Creek. At site B1, the water-level readings at the 1.0-m depth for the second location did not change, rendering the test invalid. Clay was present throughout this site and a small pocket of water may have been isolated by the clay from the aquifer when excessive pressure was applied during the slug test. Results were not obtained for the test at the 0.5-m depth for the second location at site B7 due to operational errors likely caused by application of insufficient pressure.

For the Bertrand Creek, the mean *K* values at 0.5 m ranged $0.8-351$ m d⁻¹ among the seven

sites. The mean *K* values at 1.0 m ranged 7.6–136 m d^{-1} (Figure 4, Table 1, Appendix A). These values fall within the ranges from previous studies for similar materials (Table 2).

The ANOVA tests failed to indicate a statistically significant difference in mean *K* values for the 0.5-m and 1.0-m depths at α =0.05 ($F = 4.07$, $P = 0.08$, Table 3), suggesting that sampling at the shallower level may suffice in relevant future studies in this stream. The insignificant variation between the two depths could be explained by a lack of layering in the glacial deposits in the Bertrand Creek watershed.

Both the site, and the interaction of site and depth significantly affect the means of *K*. The significant effect of site location suggests there is substantial variation along the creek. Sand and gravel glaciofluvial outwash comprises the major surficial materials. Sediments become finer towards Canada and transition to coarser materials on the US side (Scibek and Allen, 2005). The variation in lithology is a likely cause of substantial spatial variation of streambed composition and hydraulic conductivity.

For the Fishtrap Creek, the slug tests resulted in reasonable values for both the 0.5-m and 1.0-m depths for all sites. However, at site F1, location one, there was only one valid replicate at 1.0 m. The error occurred likely because insufficient pressure was applied during the test.

The mean *K* values ranged $3.7-79.7$ m d⁻¹ at 0.5 m, and $2.7-149$ m d⁻¹ at 1.0 m (Figure 4, Table 1, Appendix A). The ANOVA tests indicated that the means of *K* at 0.5 m and 1.0 m did not differ statistically at $\alpha = 0.05$ ($F = 14.3$, $P < 0.001$, Table 3), again suggesting the relative homogeneity of the streambed materials with depth. The tests showed, however, a significant difference among the sites, and the effect of interaction between site and depth was not statistically significant.

Method	Study Area	Soil Material	K , m/d	Source
Field measurements	Danube at Vienna, Austria	Gravel and sand aquifer; clogged bed layer	$0.86 - 8.63$	Sengschmitt et al., 2003
Field pumping and analytical cals.	White River, ID, USA	Channel in glacial outwash, sand and gravel	$0.86 - 86.4$	Meyer, 1978
Field pumping tests	Overijssel, Netherlands	Sands	$0.12 - 25.9$	Lamsvelt, 1985
Field pumping and analysis	Scioto River, OH, USA	silty alluvium, sand and gravel over shale bedrock	$0.3 - 69.12$	Norris 1983a,b, Nortz et al., 1994
Slug test analysis from tests bores	Grand Calumet River, ID, USA	Fine-grain sediments and fill over silty- sand aquifer	$0.06 - 604.8$	Prince et al., 1988

Table 1. Reported hydraulic conductivity values for similar riverbed materials (Calver, 2001).

Figure 4. Means of *K* from instream slug tests for 0.5-m and 1.0-m depths for (a) Bertrand Creek (B) and (b) Fishtrap Creek (F). Data are missing for location 2 at Bertrand site 1 and site 7, and Fishtrap site 1.

Depth	Mean (ft d^{-1})	Std Dev	Minimum	Maximum		
		Bertrand Creek				
0.5 _m	70.7	88.2	0.13	472.6		
1.0 _m	56.9	58.0	0.43	280.4		
		Fishtrap Creek				
0.5 _m	21.6	22.8	3.5	90.7		
1.0 _m	36.6	63.2	1.4	300.9		

Table 2. Results of instream slug tests (ft d^{-1}) at Bertrand and Fishtrap Creeks, Washington, USA.

Table 3. ANOVA results (α = 0.05) comparing means of *K* from the slug tests.

** Significant a Site*Depth, interactive effect of site and depth b Reach*Depth, interactive effect of reach and depth

This difference may be attributed to the differences in the geological materials and soils underlain the streams. Soil consists primarily of well-drained fine sandy loam to silt loam along the Bertrand Creek whereas Fishtrap Creek is covered with loam, silt loam and muck (Goldin, 1992).

Differences in land use and management practices within the two watersheds were also observed. In the Bertrand Creek watershed, land use was mainly dairy farming on the north end of our study area $(B1-B3)$, changing to berry production $(B4)$, and returning to dairy farming before the creek reaches the Nooksack River.

A buffer zone of roughly 5 m wide is established along the reach of the Bertrand Creek within our study domain. On the other hand, the surrounding land use of the Fishtrap Creek is predominantly dairy farming. The Fishtrap Creek runs through residential areas of the city of Lynden before reaching the Nooksack River. The Fishtrap Creek has large portions that are not well protected by a buffer zone compared to the Bertrand Creek. Future studies investigating how land use and management practices may affect the material composition of the streambed would be valuable.

A comparison of the slug tests at the two reaches showed a significant difference between the two reaches. Overall, the mean of *K* measured at the Bertrand Creek was larger than that measured at the Fishtrap Creek (Table 4). The ANOVA test comparing the means of *K* from the Hazen equation for the two creeks showed that there was a significant difference at the surface, but there was no statistically significant difference at the subsurface (Table 5), indicating differences in the materials of the two layers.

3.2 K Estimation from Empirical Formulas

The analysis of grain-size distribution for surface and subsurface materials in both Bertrand and Fishtrap Creeks revealed that the streambed deposits were all sand in texture (Figure 5, Appendix F) following US Department of Agriculture soil classification (Hillel, 1982). Yet the hydraulic conductivities derived from the empirical equations still varied considerably. The results have missing data at B3 and B4 for the subsurface values. Ranges of the mean *K* values at the 0.5 m depth and one standard deviation are shown in Figure 6. For the surface materials of Bertrand Creek, the mean *K* ranged 13.6–300 m d⁻¹ from the Hazen equation, and 8.7–63.2 m d⁻¹ from the Harleman equation. Both ranges fall within the literature values (Table 2). However, these two methods did not capture the extreme values as did the slug tests.

The mean *K* values from the Krumbien and Monk equation ranged $33.3-2900$ m d⁻¹, with the maximum value exceeding literature values for this type of bed material (Table 2). Foster et al. (2003), in a study of permeability of sands in the coastal region of the Baltic Sea, found that the Krumbien and Monk formula overestimated the permeability by a factor 2.6 on average. ANOVA tests indicated that the site, method, and the interaction between site and methods all have a significant effect on the means of *K* (Table 6, Appendix B). Hence, longitudinal variation in particle size and bed material composition varies substantially in Bertrand Creek.

Site F2 of the Fishtrap Creek, no viable samples were collected for the surface or subsurface material. For the remaining sites, the mean *K* for the surface materials ranged 22.1–115.0 m d^{-1} from the Hazen equation and 14.2–42.7 m d^{-1} from the Harleman equation (Figure 6). These results are within the range of typical values reported in the literature for similar bed materials(Table 2).

Table 4. ANOVA results (α = 0.05) comparing the mean values of *K* for the slug tests, Hazen, Harleman, Krumbien-Monk, and the Kozeny-Carmen equations for subsurface soil material.

** Significant

Table 5. ANOVA results of comparing means of *K* values from the Hazen method for both creeks.

** Significant

Figure 5. Particle size distribution for Bertrand and Fishtrap Creeks.

Figure 6. Empirical estimates of mean *K* for surface materials for (a) Bertrand Creek and (b) Fishtrap Creek. Note missing data at Fishtrap site 2

However, these results do not capture the extreme values obtained in the slug tests. The *K* values from the Krumbien and Monk equation ranged $602-2071$ m d⁻¹, exceeding the range of literature values (Table 2). The ANOVA test indicated that the effects of site, method, and the interaction between the site and method on the mean of *K* were all significant (Table 6, Appendix B).

For the subsurface materials in Bertrand Creek, the mean of *K* ranged 8.7–45.7 m d^{-1} , 5.6–29.3 m d⁻¹, 10.7–1050 m d⁻¹, and 14–626 m d⁻¹ from the Hazen, Harleman, Krumbien and Monk, and the Kozeny-Carman methods, respectively. The mean *K* values from the Hazen and Harlman equations were within the literature range, but the mean *K* values from both the Krumbien and Monk and the Kozeny-Carman methods exceeded the literature maximums (Table 2, Figure 7, Appendix D). The ANOVA test suggested that the means of *K* from the four empirical methods were significantly different (Table 7).

For the subsurface materials of the Fishtrap Creek, the mean of *K* ranged 7.0–47.8 m d^{-1} from the Hazen equation, 2.5–30.6 m d^{-1} from the Harleman equation, 9.6–1460 m d^{-1} from the Krumbien and Monk equation, and $46.5-16800$ m d⁻¹ from the Kozeny-Carman equation (Figure 7, Appendix D). The ANOVA test found no significant effect of either the site factor or the method factor (Table 7).

Overall, streambed *K* estimates from the Hazen and Harleman equations were similar and fell within the range of the literature values for both studied creeks, but did not capture the extremes that were revealed in the slug tests. These values are supported by the grain-size distribution measurements found in Figure 6. The Krumbien and Monk and the Kozeny-Carman equations tended to over-estimate the *K* values.

Table 6. ANOVA results (α = 0.05) comparing the mean values of *K* for the Hazen, Harleman, and Krumbien and Monk equations with the use of surface soil tests.

** Significant

a Site*Depth, interactive effect of site and depth

^bReach*Depth, interactive effect of reach and depth

Figure 7. Results of empirical estimates of *K* for (a) Bertrand Creek (B) and (b) Fishtrap Creek (F) subsurface material. Note missing data at Bertrand site

Table 7. ANOVA results (α = 0.05) comparing the mean values of *K* for the Hazen, Harleman, Krumbien-Monk, and Kozeny-Carmen equations with the use of subsurface soil materials.

** Significant
Comparison of results from the slug tests and the four empirical equations suggested that the mean *K* values at the 0.5-m depth by the five methods were significantly different (*F*-value 3.82, *p*-value 0.023) for the Bertrand Creek but were not significantly different for the Fishtrap Creek (*F*-value 1.04, *p*-value 0.418, Table 4).

3.3 Slug Test vs. Empirical Calculations

Slug tests are a common field method for estimating aquifer *K*. The applicability of the procedure to testing streambeds has been documented (Cardenas and Zlotnik, 2003; Hanrahan et al., 2005; Leek, 2006). Slug tests are useful for many studies that are focused on streambed habitat. The slug test allows for a better understanding of the connectivity to ground-water, which influences the quality and quantity of stream flow including spawning beds, and overall stream health (Meyer, 1978; Hanrahan et al., 2005). Even though the slug test method is labor-intensive and can be time-consuming if numerous tests are required this study proved to show the most representative data compared to the empirical methods.

In contrast, particle-size analysis for use in the empirical *K* estimations requires primarily sample collections with basic tools, oven-drying at 107°C, and sieving with a shaker to reduce the analysis time. The sieve shaker is the most expensive device among all needed. The particle-size analysis can be costly if it is performed by professional labs. In addition, if the streambed materials are silty or clayey in texture, a hydrometer test would be needed for a thorough analysis of particlesize distribution, which can increase both cost and time needed. The test for porosity involves mainly oven-drying and weighing, the cost of which is typically modest. Overall, the empirical methods can be less labor-demanding and less costly, and may be suited for preliminary surveying projects.

From this study, it is recommended that slug test and empirical estimations be used to

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determine *K* values when their accuracy and reliability are crucial, e.g., in detailed modeling of surface- and ground-water interactions. If *K* is not a dominant factor as compared to other physical properties e.g., in preliminary assessment for land and water resources management, the generally more cost-effective empirical procedures would be adequate.

4. SUMMARY AND CONCLUSIONS

This study investigated streambed hydraulic conductivity of two salmon-habitat streams, Bertrand Creek and Fishtrap Creek, in the state of Washington, Pacific Northwest, USA. The two streams are typical examples in the region where water levels in stream are impacted by seasonal water shortages during summer months. Streambed *K* was measured using slug tests and analyzed following the Bouwer-Rice method. A second, low-cost approach of empirical formulas based on measurement of representative particle-size distribution was also used to estimate *K*. Comparisons were made of mean *K* values within and between the two streams from the slug tests, and of mean *K* values determined from the slug tests vs. the empirical estimation. Major findings from this study included:

- *K* values from the slug tests at the two streams of Bertrand Creek and Fishtrap Creek fell in the literature range for similar streambed materials.
- There was no significant difference between mean *K* values between the 0.5-m and 1.0-m depths from the slug tests for both Bertrand Creek and Fishtrap Creeks, suggesting sampling at the shallower level may suffice in relevant future studies.
- The mean of *K* for Bertrand Creek from the combined surface and subsurface samples was significantly higher than that of the Fishtrap Creek due to the difference in geological deposits at the two sites.

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- For the surface and subsurface materials of both Bertrand and Fishtrap Creeks, estimates of *K* from the Hazen and Harleman showed no significant difference. However, the Krumbien and Monk and the Kozeny-Carmen equations resulted in much higher estimations than the previous two methods and exceeded literature values. Hence, the Hazen and Harleman equations may be used for future studies and the Krumbien andMonk and the Kozeny-Carmen equations may not be applicable to this region.
- Comparison of mean *K* values estimated for the different sites using the Hazen method showed that there was a significant difference along the studied reach of Bertrand Creek but not along the studied reach of Fishtrap Creek.
- Comparison of results from the slug tests and empirical estimations indicated that the mean K values at the 0.5-m depth by the five methods were significantly different for the Bertrand Creek but were not significantly different ere for the Fishtrap Creek.

In determining the appropriate method for obtaining *K* in relevant future studies, we recommend the following. If accuracy and reliability of the *K* values are desired for a detailed study, e.g., development of models of surface- and ground-water interaction, the slug test method is well suited. If *K* is not a dominant factor in the study, then the empirical method would be a more cost-effective approach.

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APPENDICES

APPENDIX *A*

Calculations of *K* **from the instream slug tests, Bertrand Creek and Fishtrap Creek, Washington State, USA**

Estimation of K from a Slug Test Using Four Different Approaches (based on notes from Li Wang, PhD student, BSYSE, WSU)

Four slightly different approaches were used to solve for *K* of a single slug test following the Bouwer and Rice equation (Bouwer and Rice, 1976): curve matching, least-squares, and arithmetic and harmonic means of *K* calculated for each time step for the entire data set.

1. Curve matching

In Bouwer and Rice (1976), a straight line was fitted with measured data $ln(H_t)$ vs *t*. An arbitrary point (t, H_t) on the line together with the intercept H_0 was selected to calculate *K* using the following equation

$$
K = \frac{r^2 \ln(R_e/R)}{2L_e} \frac{1}{t} \ln\left(\frac{H_0}{H_t}\right)
$$
 (A.1)

K was obtained by curve matching, i.e., by automatically fitting the equation

$$
\frac{H_t}{H_0} = \exp(bt)
$$
 (A.2)

to the measured data, where $b = -\frac{K}{C}$ and $C = \frac{r^2 \ln(R_e/R)}{2L_e}$. The fitted coefficient *b* was then used to

calculate $K = -bC$.

If, after visual inspection and selection from the original data, we find that the point (t_0, H_0) still departs from the major trend of the remaining data, we may decide to use another form of the Bouwer-Rice equation

$$
\frac{H_t}{H_0} = \exp(bt) \tag{A.3}
$$

2. Least-squares

If we use equation $(A.2)$ and assume a reasonable K for an initial guess, H_t can be calculated for every pair of data corresponding to time 0–*t*. The *K* value of the slug test can then be calculated by comparing the predicted with the measured H_t 's and minimizing their MSE (mean squared error). This can be done by using the Solver function of Microsoft Excel.

3 and 4. Arithmetic and Harmonic means

Theoretically, *K* can be calculated by using any two points of measured data (t_1, H_1) and (t_2, H_2) or $(t_1, \frac{H_1}{H_0})$ and $(t_2, \frac{H_2}{H_0})$ during a single slug test. The equation is

$$
K = \frac{C}{t_2 - t_1} \ln\left(\frac{H_1}{H_2}\right) \tag{A.4}
$$

When $t_1 = 0$, let $t = t_2$, $H_0 = H_1$, and $H_t = H_2$. Then, the above equation resumes the form of equation (A.1).

A series of *K* values can be calculated using the selected data (t_0, H_0) and the data points thereafter. If the resultant *K*'s exhibit a declining trend with time, the harmonic mean should be used to obtain an average *K*; otherwise, the arithmetic mean may be used.

Additional notes that are crucial and helpful are included below.

- In equation $(A.1)$, point (t_0, H_0) is very important for the result of *K*, since it is used as many times as the number of data points, while other points are used only once. From field experience, the recovery curve is likely to have a larger slope or fluctuations at the beginning part. Hence, one should check this part carefully. Setting different points as the (t_0, H_0) point and only using this point and the data points after it would favor different stages of the slug test or different influence areas.
- In using equation (A.2), if we force the intercept $a = 1$, the (t_0, H_0) point is favored. The resultant equation is equivalent to the Bouwer-Rice (1976) equation.
- In using equation (A.3), if we fit both coefficients *a* and *b* in the equation, the main trend of the data set is favored. Doing so eliminates the dominant influence of the (t_0, H_0) point. Such approach is equivalent to the one used by Bouwer and Rice (1976), in which the fitted intercept H_0 was used instead of the theoretical value.
- Visual data inspection and selection is important in perhaps any slug test analysis.
- The term $\frac{K}{C}t$ may be considered as the dimensionless *K*.
- Equation (A.4) is similar to that of the falling-head method in measuring saturated hydrauclic conductivity typically used in soil physics.
- By comparing the resultant *K* vs *t*, it may also be helpful to understand the behavior of the heterogeneous porous media surrounding the piezometer. Although the Bouwer and Rice equation (1976) is generally applied under conditions of homogeneous porous media, heterogeneity naturally occur at any spatial scale. It is only a matter of how heterogeneous the porous media are.

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Example 1. Bertrand Creek site one, location one, 0.5-m depth, replicate one. This is an example of a valid slug test and adequate curve fitting. Listed are only the data points for the beginning of the test.

					Slug Test --- Estimate K						
					Inputs (m)		Solver---> K	$1.50E-05 \, \rm{m/s}$		\leftarrow --run solver	
	$H(m)-$ (Assume)	6.00	Radius of the well casing	$\bf r$	0.033		2. Curve \leftarrow \geq Co	-0.0053			
	Lw(m)	1.09	Distance for measuring $\rm K$	Re	0.103		K	$1.67E-05 \, \text{m/s}$			
	Transducer Position above bottom of pizometer (m)	0.10	Radius of the gravel envelop	${\bf R}$	0.043	1.20 1.00 0.80			Measured Calculated Expon. (Measured)		
			length of the screen	Le	0.152	Height (cm) 0.60 0.40					
$\mathbf X$	\mathbf{A}	$\, {\bf B}$	Drawdow m at $t=0$	H ₀	0.796	0.20 0.00					$y = e^{-0.0053x}$ $R^2 \neq 0.9303$
0.553	1.75	0.23	Initial Water level	DOW	0.992		0.00 200.00	600.00 400.00 Time (s)	800.00	1000.00	
			Constant	$\mathbf C$	0.003	$\overline{3}$. Arithmeti $\mathbf c$ Average $\bf K$	4. $n\sqrt{\Sigma(1/Ki)}$				
				sum()	2.707	1.70E-05	1.60E-05				
	t(s)	Ht (cm)	measured H_t/H_{t0}	Calculated H_t/H_{t0}	$(E-F)^2$	Ki					
	0.00	79.63	1.00	1.00	0.00						
	0.25	78.52	0.99	1.00	0.00	1.8E-04	5.6E+03				
	0.50	78.00	0.98	1.00	0.00	1.3E-04	7.7E+03				
	0.75	76.36	0.96	1.00	0.00	1.8E-04	5.7E+03				
	1.00	75.10	0.94	1.00	0.00	1.8E-04	5.4E+03				

Example 2. Fishtrap Creek, site one, location one, 0.5-m depth, second replicate. This is an example of a valid slug test and adequate curve fitting. Listed are only the data points for the beginning of the test.

				Slug Test --- Estimate K							
				Inputs (m)				Solver--->9.03E-05			
	$H(m)-$ (Assume)	6	Radius of the well casing	$\bf r$	0.03302		2. Curve -	-0.234			
	Lw(m)	0.957	Distance for measurin g K	Re	0.1015		$\bf K$	7.25E-04			
	Transducer Position above bottom of pizometer (m)	0.1	Radius of the gravel envelop	${\bf R}$	0.04267	1.20 1.00 0.80				Measured Calculated Expon. (Measured)	
			length of the screen	Le	0.1524	Head Height (cm) 0.60 0.40				$y = e^{-0.0234x}$ $R^2 = 0.9221$	
$\mathbf X$	A	B	Drawdo wm at $t=0$	H ₀	0.82236	0.20 0.00	$\mathbf 0$ 50	100	150	200 250	300
0.553	1.747	0.233	Initial Water level	DOW	0.857				Time (s)		
			Constant	$\mathbf C$	0.0031	3. Arithmeti \mathbf{c} Average $\bf K$	4. $n\sqrt{\Sigma(1/Ki)}$				
				sum()	0.0931	1.18E-01	1.71E-03				
	t(s)	Ht (cm)	measured Calculate H_t/H_{t0}	$d H_t/H_{t0}$	$(E-F)^2$	Ki					
	$\pmb{0}$	82.236	1.00	1.00	0.00						
	0.25 0.5	81.056 83.047	0.99 1.01	0.99 0.99	0.00 0.00	1.00 0.99	1 1.007307				

			Slug Test --- Estimate K									
				Inputs (m)			Solver--->	7.70E-05 m/s		<---run solver		
	$H(m)$ -- (Assume λ	6	Radius of the well casing	$\bf r$	0.03302		2. Curve --	0.026			<---type Coef. in Fig. 1	
	Lw(m)	0.549	Distance for measurin g K	Re	0.0958		$\bf K$	7.53E-05 m/s				
	Transduc er Position above bottom of pizometer (m)	0.1	Radius of the gravel envelop	$\mathbf R$	0.04267	1.20 1.00 0.80	$y = 1.0188e^{0.026x}$					
			length of the screen	Le	0.1524	Height (cm) 0.60 0.40	$R^2 = 0.9232$ Measured Calculated					
$\mathbf X$	A	$\, {\bf B}$	Drawdo wm at $t=0$	${\rm H}0$	0.0495	0.20 0.00	Expon. (Measured) 3 $\overline{2}$ $\overline{4}$ $\mathbf 0$ $\mathbf{1}$					5
0.553	1.747	0.233	Initial Water level	DOW	0.449				Time (s)			
			Constant	C	0.00289	3. Arithmeti $\mathbf c$ Average $\bf K$	4. $n/\Sigma(1/Ki)$					
				sum()	0.4000	1.16E-04	1.09E-04					
	t(s)	Ht (cm)	measured Calculate H_t/H_{t0}	d H_t/H_{t0}	$(E-F)^2$	Ki						
	$\mathbf 0$	4.95	1.00	1.00	0.00							
	0.25 0.5	5.02 5.1	1.01 1.03	0.99 0.99	0.00 0.00	1.6E-04 1.7E-04	$6.2E + 03$ 5.8E+03					

Example 3. Bertrand Creek, site one, location two, 1.0-m depth, replicate one. This is an example of a non-usable slug-test measurement. Listed are only the data points for the beginning of the test.

Estimated K for Streambeds of Bertrand Creek and Fishtrap Creek

Bertrand Creek Slug Tests

Fishtrap Creek Slug Test Results

 $6.134E+00$

FT31-1-1 6.61E+00 7.932E+00 7.154E+00 8.130E-05 FT31-1-2 5.95E+00 6.800E+00 6.420E+00 7.310E-05

Appendix B Estimation of K from four empirical formulas, Bertrand Creek and Fishtrap Creek, Washington State, USA: Examples

Bertrand Creek

Harleman		$k=(6.54*10^{-4})d_{102}$		$K=k^*\rho_w^*g/\mu$				
	$d_{10\,(\text{cm})}$	k (cm2)	K (cm/s)	m/s	m/day			
	0.010	6.082E-08		0.006	5.955E-05	$5.145E+00$		
Krumbien and Monk			$k = 760 \times d^2e^{-1.31\sigma}$					
d geometric mean (mm)	σ (mm)	k (darcy)	K (cm/s)		m/s	m/day		
0.394	0.243	85.673		0.082	0.001	7.117E+01		
Kozeny-Carmen Bear					$K=(\rho_{w}g/\mu) * (n^{3}/(1-n)^{2}) * (d^{2m}/180)$			
	Used Geometric Mean							
$n = f$	d (cm)		K (cm/s)		m/s	m/day		
0.322	0.039			6.139E-02	6.139E-04	$5.304E + 01$		

Estimated K for streambeds of Bertrand and Fishtrap Creek.

Appendix C Comparison of Means of K from the Instream Slug Tests Using the GLM Procedure in SAS

Comparison of means of K for the 0.5-m and 1.0-m depths for Bertrand Creek

SAS codes

DATA A; INFILE 'C:\JOAN\CM\BCSLUG.PRN'; INPUT SITE DEPTH \$ K @@; PROC PRINT; PROC GLM; CLASS SITE DEPTH; MODEL K = SITE DEPTH SITE*DEPTH; MEANS DEPTH/TUKEY ALPHA=0.01; MEANS DEPTH/TUKEY; MEANS DEPTH/TUKEY ALPHA=0.1; RUN;

SAS results

Class Level Information

Dependent Variable: K

Tukey's Studentized Range (HSD) Test for K

NOTE: This test controls the Type I experiment wise error rate, but it generally has a higher Type II error rate than REGWQ.

NOTE: Cell sizes are not equal.

Means with the same letter are not significantly different.

Comparison of means of K for the 0.5-m and 1.0-m depths for Fishtrap Creek

SAS codes

DATA A; INFILE 'C:\JOAN\CM\FTSLUG.PRN'; INPUT SITE DEPTH \$ K @@; PROC PRINT; PROC GLM; CLASS SITE DEPTH; MODEL K = SITE DEPTH SITE*DEPTH; MEANS DEPTH/TUKEY ALPHA=0.01; MEANS DEPTH/TUKEY; MEANS DEPTH/TUKEY ALPHA=0.1; RUN;

SAS results

Tukey's Studentized Range (HSD) Test for K

NOTE: This test controls the Type I experiment wise error rate, but it generally has a higher Type II error rate than REGWQ.

NOTE: Cell sizes are not equal

Means with the same letter are not significantly different

Comparison of means of K between Bertrand Creek and Fishtrap Creek

SAS codes

DATA A; INFILE 'C:\JOAN\CM\SLUG.PRN'; INPUT REACH SITE DEPTH \$ K @@; PROC PRINT; PROC GLM; CLASS REACH DEPTH; MODEL K = REACH DEPTH REACH*DEPTH; MEANS DEPTH/TUKEY ALPHA=0.01; MEANS DEPTH/TUKEY; MEANS DEPTH/TUKEY ALPHA=0.1; RUN;

SAS results

 $C7$

Dependent Variable: K

Tukey's Studentized Range (HSD) Test for K

 NOTE: This test controls the Type I experiment wise error rate, but it generally has a higher Type II error rate than REGWQ.

 NOTE: Cell sizes are not equal. Means with the same letter are not significantly different.
Tukey Grouping Mean N REACH Mean N REACH

Appendix D Comparison of Means of *K* from Empirical Formulas Using the GLM Procedure in SAS

Comparison of means of K from Hazen, Harleman, and Krumbien and Monk equations for the streamed surface samples for Bertrand Creek.

SAS Codes

DATA A; INFILE 'C:\JOAN\CM\BSREMP.PRN'; INPUT SITE METHOD \$ K @@; PROC PRINT; PROC GLM; CLASS SITE METHOD; MODEL K = SITE METHOD SITE*METHOD; MEANS METHOD/TUKEY ALPHA=0.01; MEANS METHOD/TUKEY; MEANS METHOD/TUKEY ALPHA=0.1; RUN;

SAS results

Dependent Variable: K

Tukey's Studentized Range (HSD) Test for K

NOTE: This test controls the Type I experiment wise error rate, but it generally has a higher Type II error rate than REGWQ.

Means with the same letter are not significantly different.

Comparison of means of K from Hazen, Harleman, and Krumbien and Monk equations for the streamed surface samples for Fishtrap Creek

SAS codes

DATA A; INFILE 'C:\JOAN\CM\FTSREMP.PRN'; INPUT SITE METHOD \$ K @@; PROC PRINT; PROC GLM; CLASS SITE METHOD; MODEL K = SITE METHOD SITE*METHOD; MEANS METHOD/TUKEY ALPHA=0.01; MEANS METHOD/TUKEY; MEANS METHOD/TUKEY ALPHA=0.1; RUN;

SAS results

Dependent Variable: K

Tukey's Studentized Range (HSD) Test for K

NOTE: This test controls the Type I experiment wise error rate, but it generally has a higher Type II

Means with the same letter are not significantly different.

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Comparison of means of K from Hazen, Harleman, Krumbien and Monk, and Kozeny-Carmen equations for the streamed subsurface (~0.5 m) samples for Bertrand Creek

SAS codes

DATA A; INFILE 'C:\JOAN\CM\BSSEMP.PRN'; INPUT SITE METHOD \$ K; PROC PRINT; PROC GLM; CLASS SITE METHOD; $MODEL K = SITE METHOD;$ RUN;

SAS results

Dependent Variable: K

Sum of
DF Squ Source DF Squares Mean Square F Value Pr > F

D₅

Model			1023192.577	146170.368	3.05	0.0432	
Error		12	574269.055	47855.755			
Corrected Total		19	1597461.632				
	R-Square		Coeff Var	Root MSE	K Mean		
	0.640512		120.3238	218.7596	181.8090		
Source		DF	Type I SS	Mean Square F Value $Pr > F$			
SITE		4	400217.6721	100054.4180	2.09	0.1451	
METHOD		3	622974.9045	207658.3015	4.34	0.0274	

Comparison of means of K from Hazen, Harleman, Krumbien and Monk, and Kozeny-Carmen equations for the streamed subsurface (~0.5 m) samples for Fishtrap Creek

SAS Code

DATA A; INFILE 'C:\JOAN\CM\FTSSEMP.PRN'; INPUT SITE METHOD \$ K; PROC PRINT; PROC GLM; CLASS SITE METHOD; $MODEL K = SITE METHOD;$ RUN;

SAS Results

Obs SITE METHOD K

Class Level Information

Class Levels Values

Comparison of means of K using Hazen equation for surface samples, Bertrand Creek and Fishtrap Creek

SAS codes

DATA A; INFILE 'C:\JOAN\CM\HAZENSr.PRN'; INPUT REACH SITE METHOD \$ K @@; PROC PRINT; PROC GLM; CLASS SITE METHOD; MODEL $K = REACH;$ MEANS REACH/TUKEY ALPHA=0.01; MEANS REACH/TUKEY; MEANS REACH/TUKEY ALPHA=0.1; RUN;

SAS results

 NOTE: This test controls the Type I experiment wise error rate, but it generally has a higher Type II error rate than REGWQ.

Means with the same letter are not significantly different.

Tukey Grouping Mean N REACH

Comparison of means of K using Hazen equation for subsurface samples, Bertrand Creek and Fishtrap Creek

SAS codes

DATA A; INFILE 'C:\JOAN\CM\HAZENSs.PRN'; INPUT REACH SITE METHOD \$ K @@; PROC PRINT; PROC GLM; CLASS SITE METHOD; $MODEL K = REACH;$ RUN;

SAS results

Obs REACH SITE METHOD K

Class Level Information

Dependent Variable: K

Source DF Type I SS Mean Square F Value Pr > F

REACH 1 263.5384896 263.5384896 0.87 0.3782

Appendix E Comparison of Means of *K* at the 0.5-m Depth from Five Methods (Slug Test and Four Empirical Formulas) Using the GLM Procedure in SAS

SAS Code

DATA A; INFILE 'C:\CM\FTAll.PRN'; INPUT SITE METHOD \$ K; PROC PRINT; PROC GLM; CLASS SITE METHOD; $MODEL K = SITE METHOD;$ RUN;

SAS Results

Dependent Variable: K

Error 16 171361131.5 10710070.7
Corrected Total 24 268701405.9 24 268701405.9

Appendix F Calculations of Particle-size Distribution for the Subsurface Bed Materials of the Bertrand and Fishtap Creeks

Bertrand Creek

Fishtrap Creek

Appendix G Long-term Climate Data for the Study Site from NOAA NCDC

Climate data was downloaded from NOAA-NCDC for Bellingham, WA Airport Station and saved in a Microsoft Excel file. The raw data is on sheet RAW. This sheet contains monthly mean max and min temperatures filtered from the raw data. A total of 53 years (1949–2001) of data were downloaded; however, only 47 years of data were used in analysis and those years with considerable missing data were not included. Temperature was originally in Fahrenheit and was converted to degree Celsius in the manuscript.

Mean

Mean

Min January Feburary March April May June July August September October November December

1961 41.8 44.4 46.1 48 54.3 60.8 64.6 65.2 56 48.6 40.6 39.1 1962 37.2 42 41.1 49.2 51.1 57.7 61.3 60.7 57 51.5 46.3 42

